



University of
New Hampshire



Minnesota Asphalt
Pavement Association

Balanced Mix Design Approach

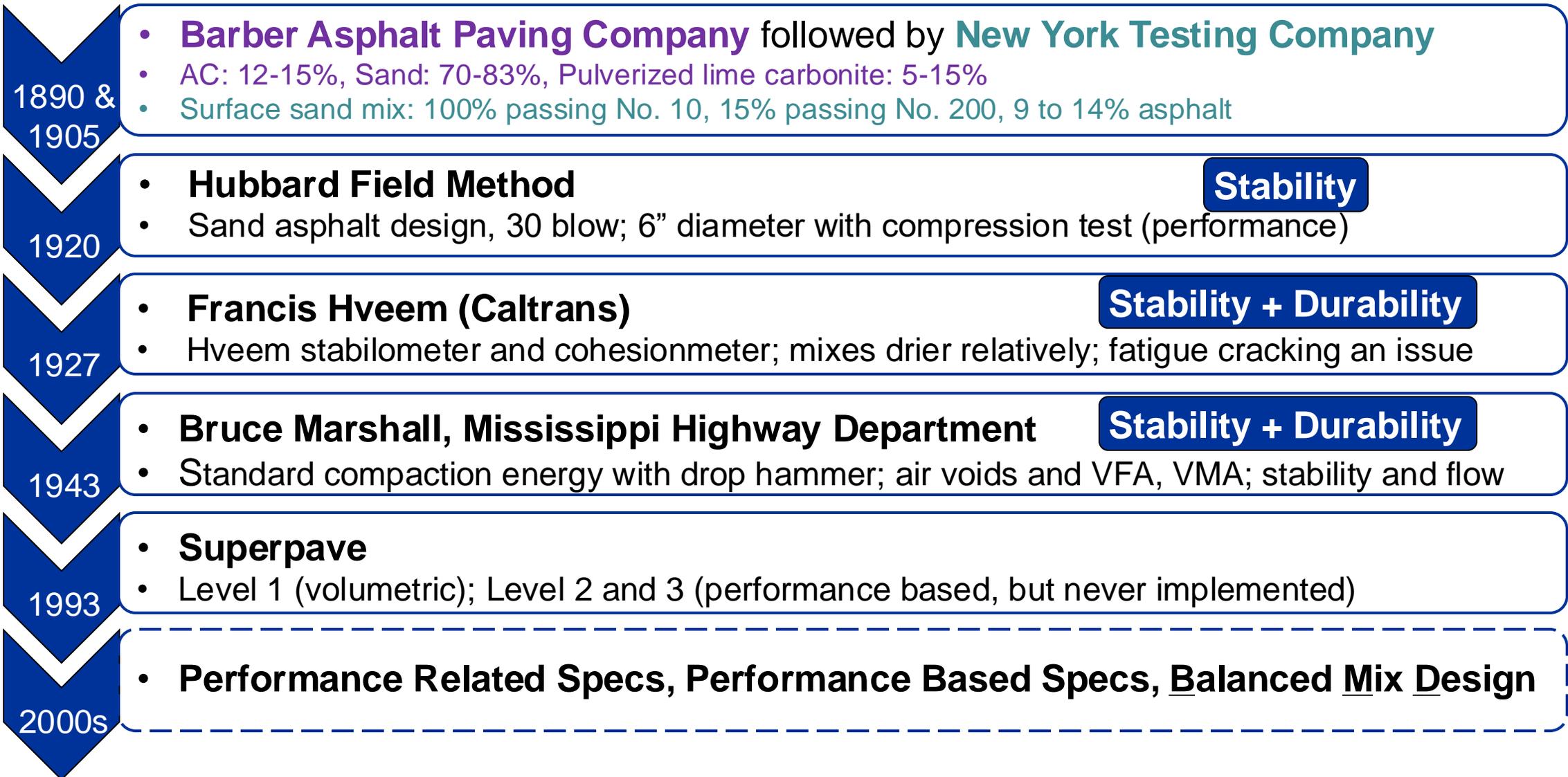
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Co-authors: Shubham Modi and Jo E. Sias

University of New Hampshire

2024 MN Asphalt Conference, St. Cloud

History of Asphalt Mix Design Process



Outline

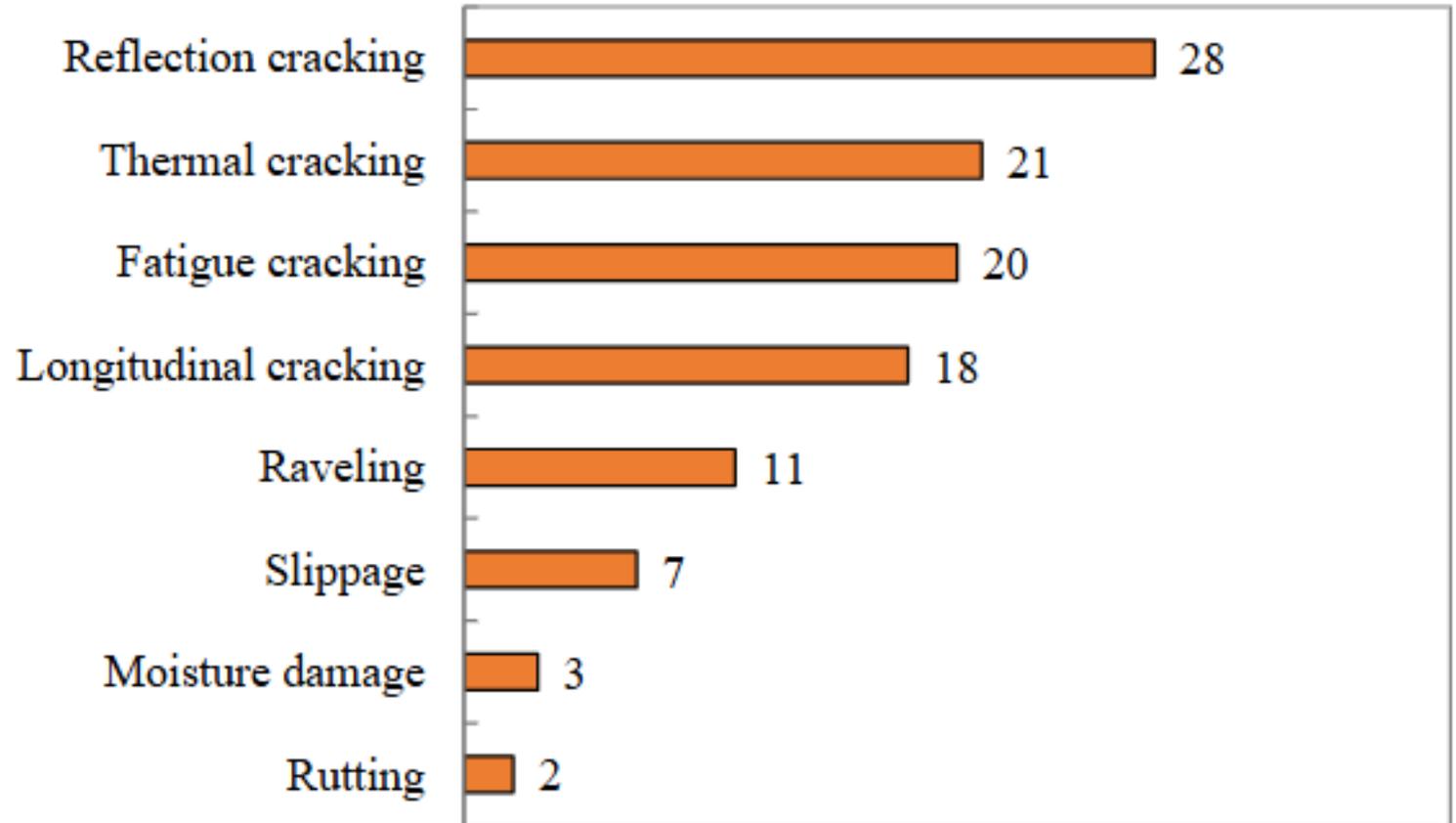
- Why we need new mix design approach?
- What is balanced mix design?
- Who is implementing BMD?
- BMD Case Studies:
 - Illinois DOT
 - Wisconsin DOT
 - Alabama DOT
 - Virginia DOT
- Critical Considerations and Research Efforts on BMD

Why We Need a New Mix Design Approach?



What Type of Distresses are Occurring Nowadays?

- **51 responses** – 34 US states and 2 Canadian provinces
- Top three:
 1. **Reflection cracking**
 2. **Thermal cracking**
 3. **Fatigue cracking**
- Asphalt community is less concerned about rutting



Current Mix Design Requirements

- Problems:
 - Dry mixes (**Cracking**)
 - Volumetric properties (V_a and AFT [V_{be}]) **lacks quality insights** into the binder and mixtures
 - Does not adequately account for **modern materials** like recycled materials, warm-mix additives, polymers, rejuvenators
- Potential Solutions:
 - **Recognize** performance issues related to dry mixes in some areas
 - Increase understanding of the **factors affecting performance**
 - **Design for performance**

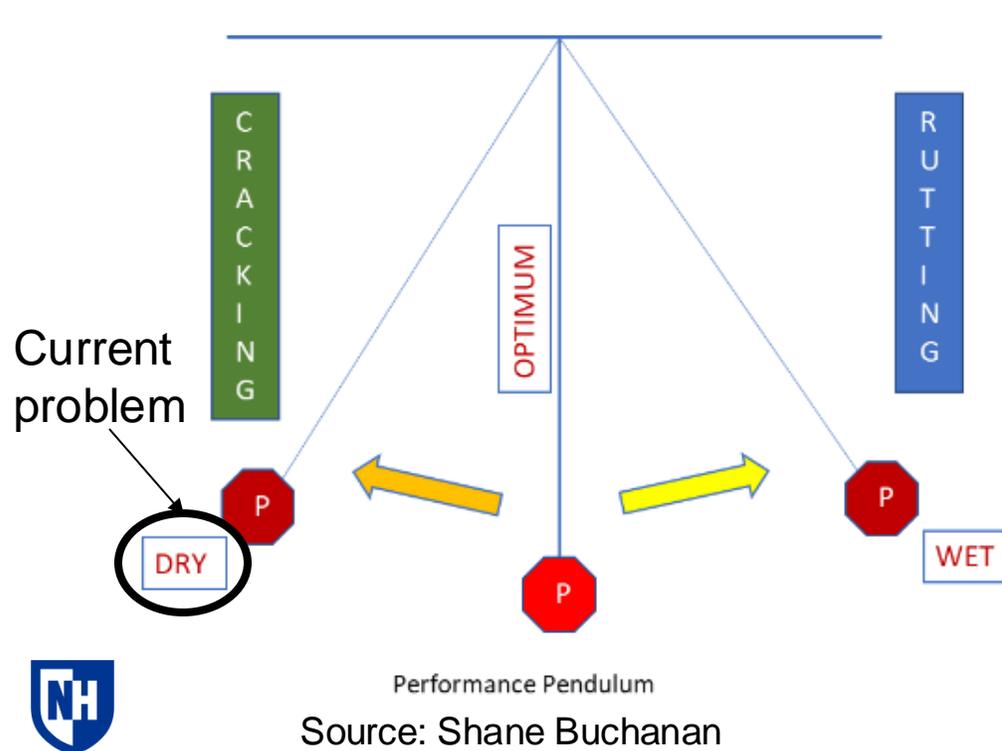


What is Balanced Mix Design (BMD)?



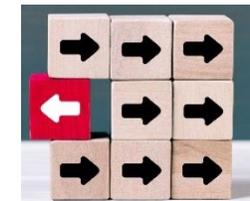
BMD – Definition and Advantages

- Asphalt mix design using performance tests on appropriately conditioned specimens that address multiple modes of distress taking into consideration mix aging, traffic, climate, and location within the pavement structure (AASHTO PP 105)

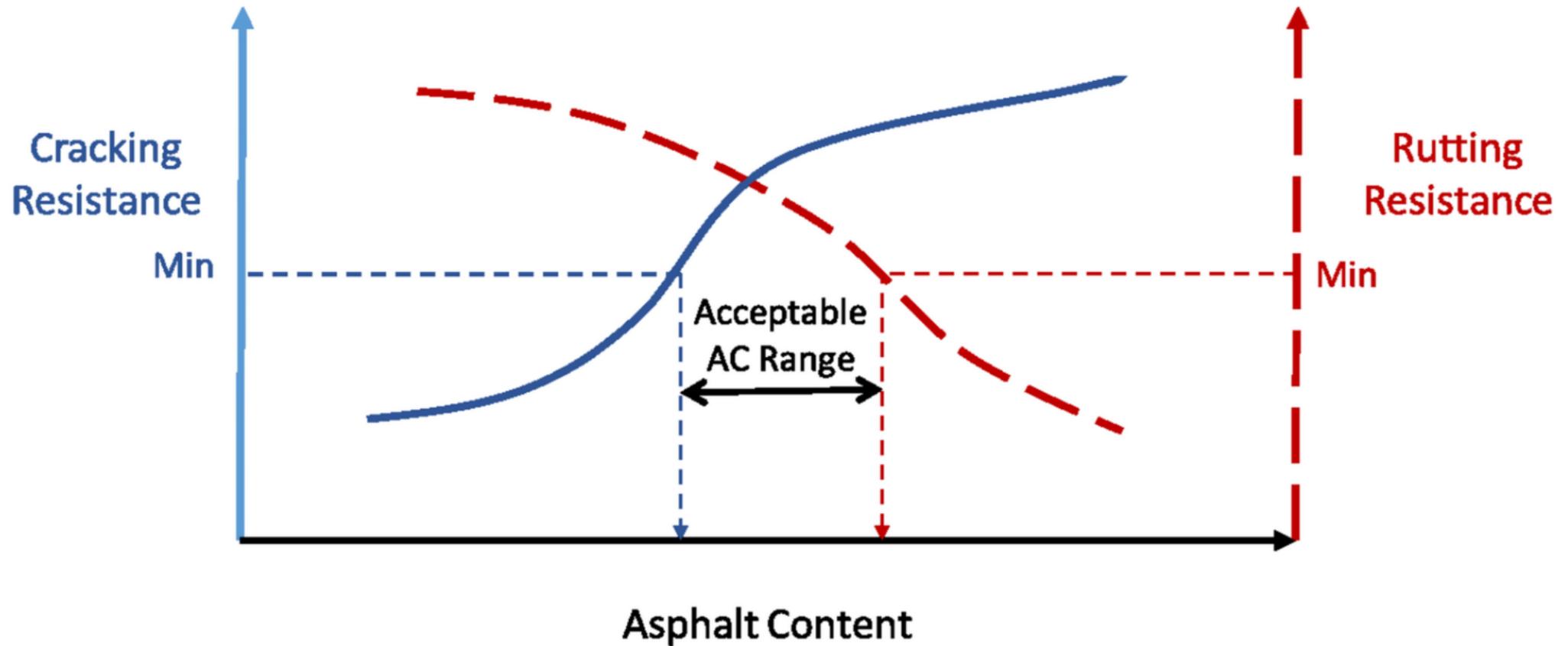


?

- Ensure performance
- Enable innovation
- Economic optimization



Concept of BMD



- Mix design to balance the major distresses such as rutting and cracking

BMD Approaches

A

Volumetric
Design

Perf. Test
Criteria

Volumetric Design
with Performance
Verification

B

Initial
Volumetrics

Perf. Tests @
Vol. Opt. AC
 $\pm 0.6\%$

Volumetric Design
with Performance
Optimization

C

Initial
Volumetrics

Perf. Tests @
Adjust %AC &
Components

**Performance
Modified** Volumetric
Design

D

No Volumetric
Requirements

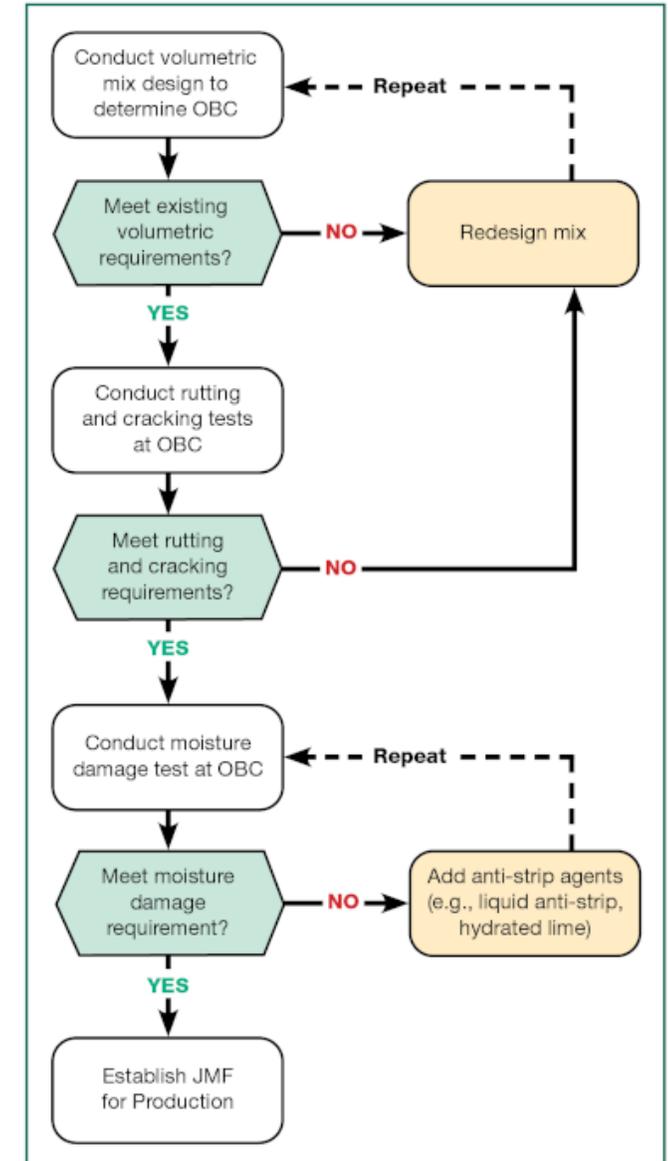
Perf. Tests @
Adjust Proportion
and Components

**Performance
Design**

Approach - A

Volumetric Design with Performance Verification

- Features:
 - Addition of performance testing to Superpave
 - **Easiest** from an Agency perspective
 - Uses current QCs for acceptance
 - Cost, time, and carbon footprint increase
 - Only option to increase mix VMA (Iterative mix design)
 - Limits RAP and other recycled materials

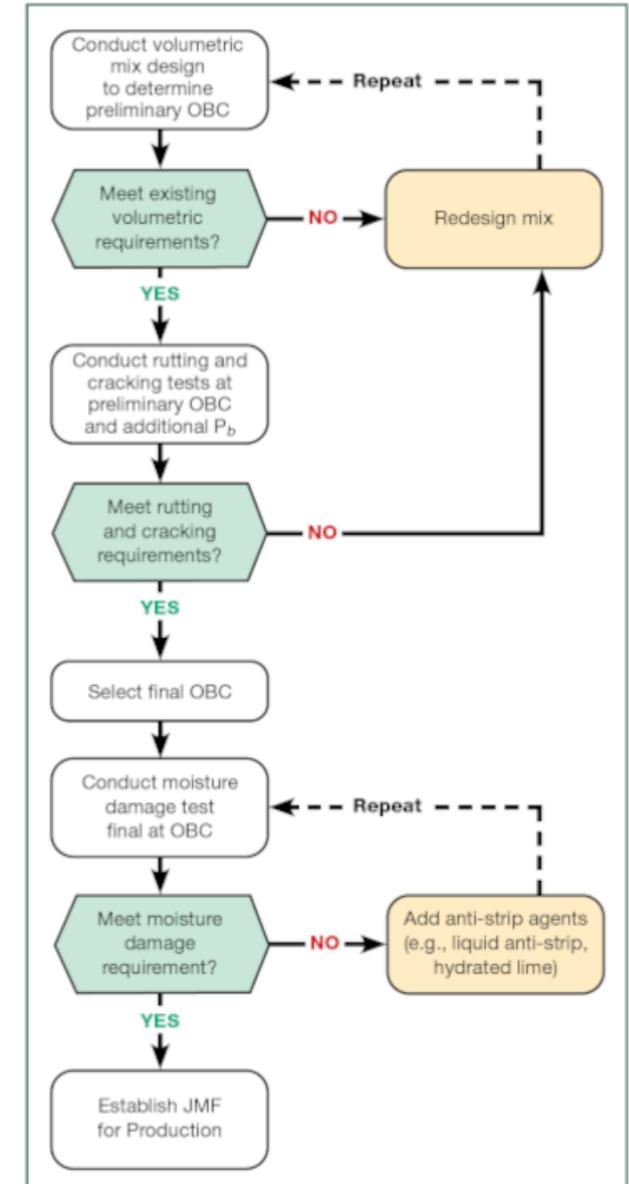


Most conservative
Lowest innovation potential

Approach - B

Volumetric Design with Performance Optimization

- Features:
 - Addition of performance test to Superpave
 - **Easy** from an Agency perspective
 - Current QCs can be used for acceptance, with a shift in target air voids
 - Air void and VFA criteria must be relaxed
 - Cost, time, and carbon footprint increase
 - Only binder content can be adjusted Limits RAP and other recycled materials



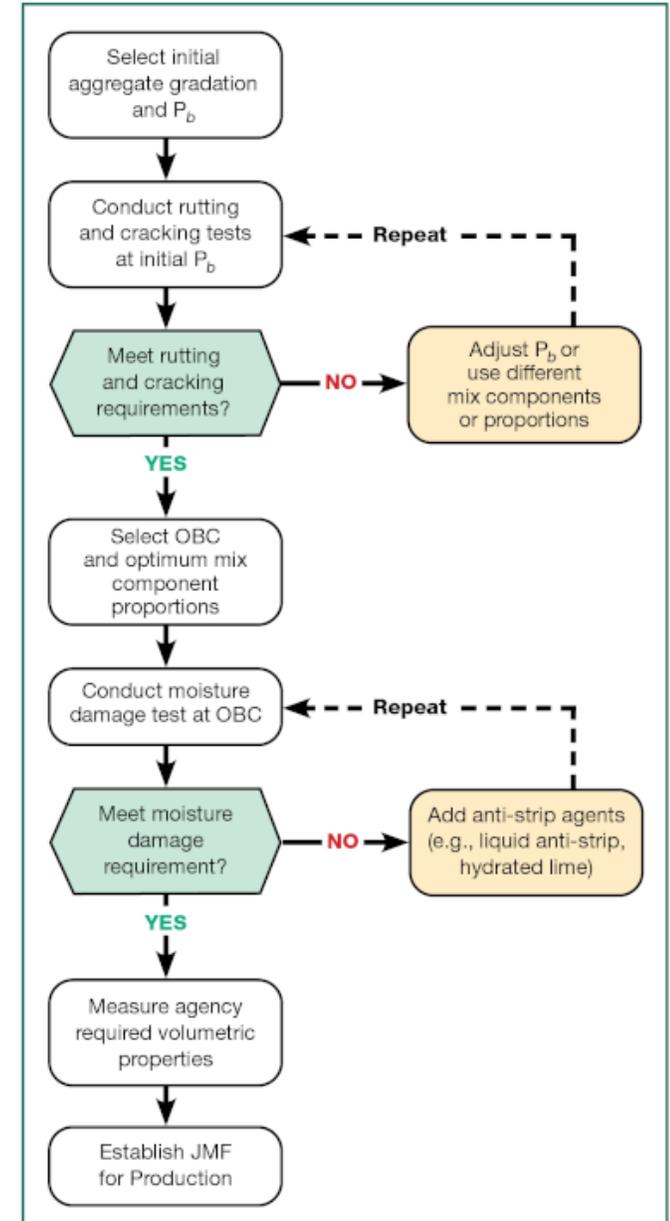
Slightly more flexible
Limited innovation potential

Approach - C

Performance Modified Volumetric Design

- Features:
 - Volumetric criteria can be relaxed or eliminated if performance criteria are satisfied
- Agencies are free to:
 - Utilize a binder grade of their choice and alter the gradation
 - Increase utilization of recycled materials
 - Use other modern-day materials (recycling agents, fibers, innovative materials)

Less conservative
Medium innovation potential

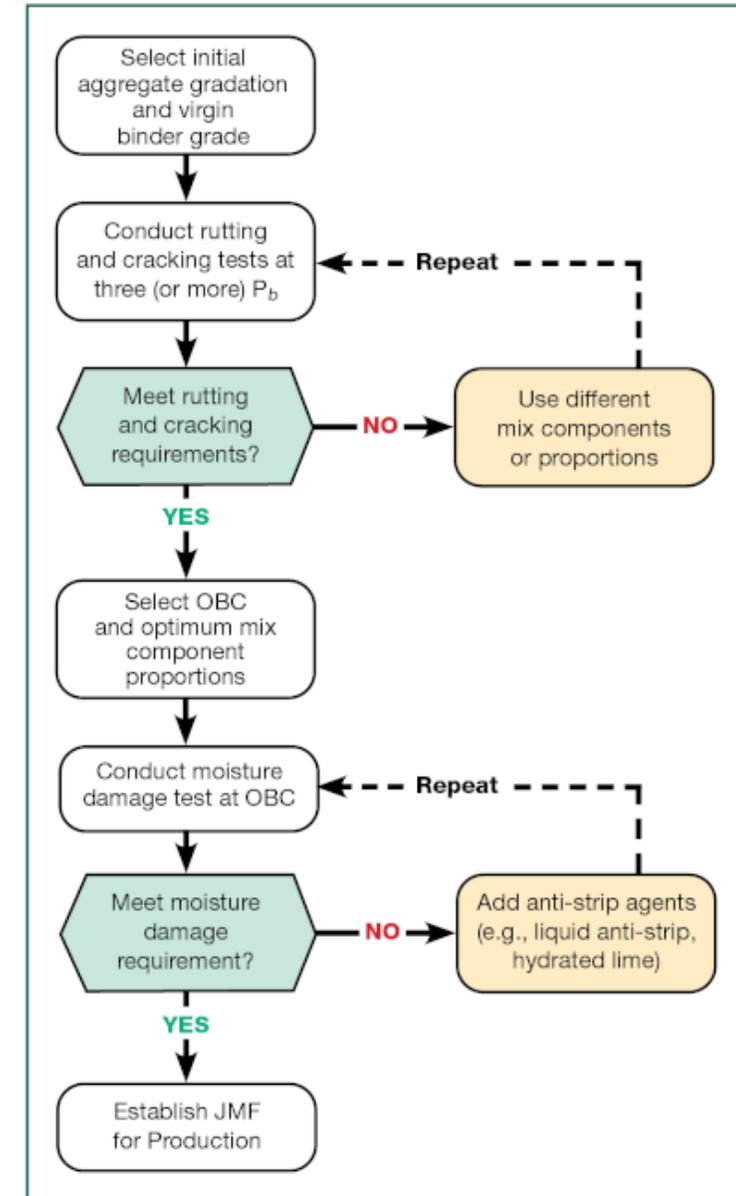


Approach - D

Performance Design

- Features:
 - No criteria for volumetric (serves as a guide)
 - Only the performance criteria must be satisfied
- Agencies are free to:
 - Utilize a binder grade of their choice and alter the gradation
 - Increase utilization of recycled materials
 - Use other modern-day materials (recycling agents, fibers, innovative materials)

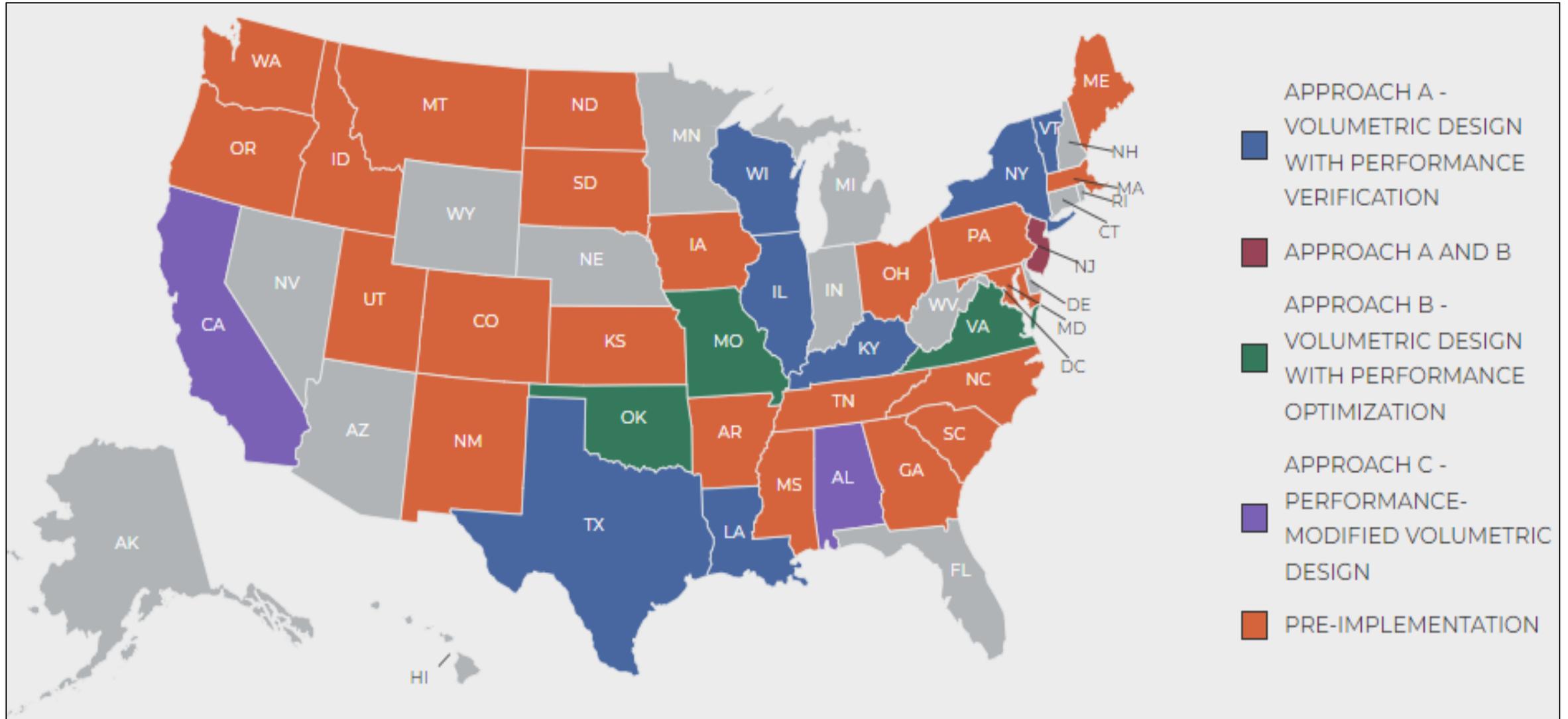
Least conservative
Highest innovation potential



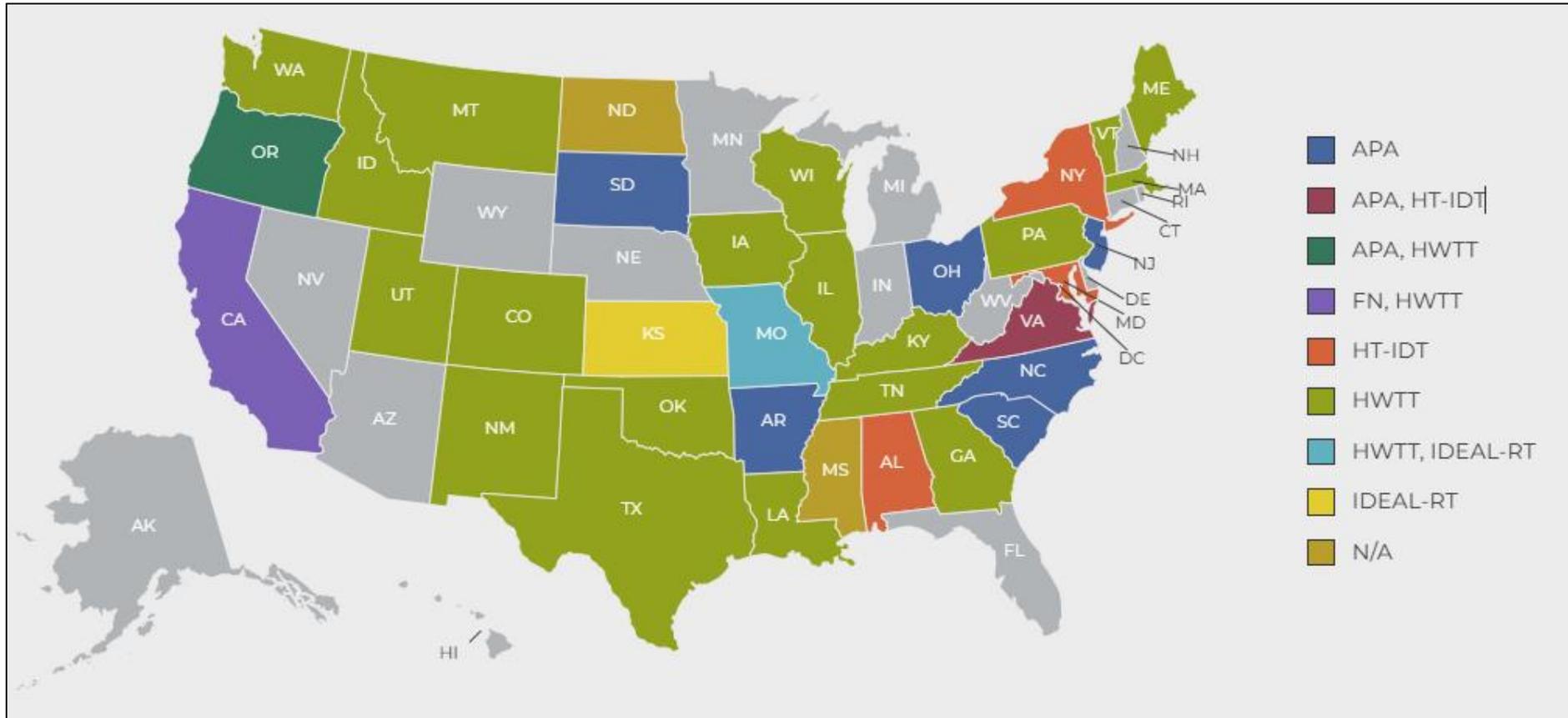
Who is implementing BMD?



BMD Implementation Status – Approaches

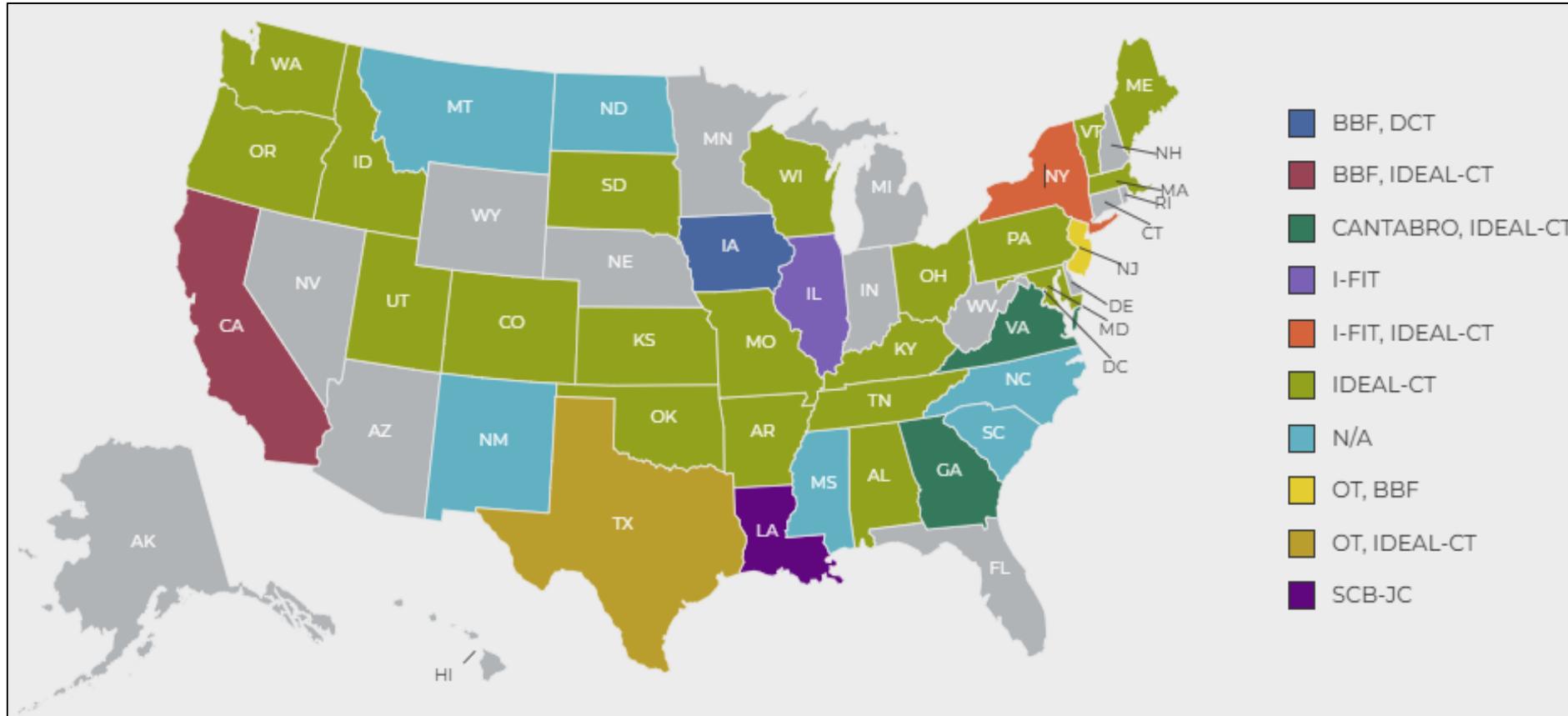


BMD Implementation Status – Rutting Tests



Test	No. of States
HWTT	22
APA	8
HT-IDT	4
FN	1
IDEAL-RT	2

BMD Implementation Status – Cracking Tests



Test	No. of States
IDEAL-CT	25
I-FIT	2
BBF	3
Cantabro	2
OT	2
SCB-J _c	1

BMD Case Studies



Illinois

- Problem Statement:
 - VMD with inadequate cracking resistance

BMD Test Parameter	Average	IDOT BMD Spec. (Average)	Pass/Fail
HWTT Rut Depth at 15,000 Passes (mm)	2.0	≤12.5	Pass
I-FIT FI	5.4	≥8.0	Fail



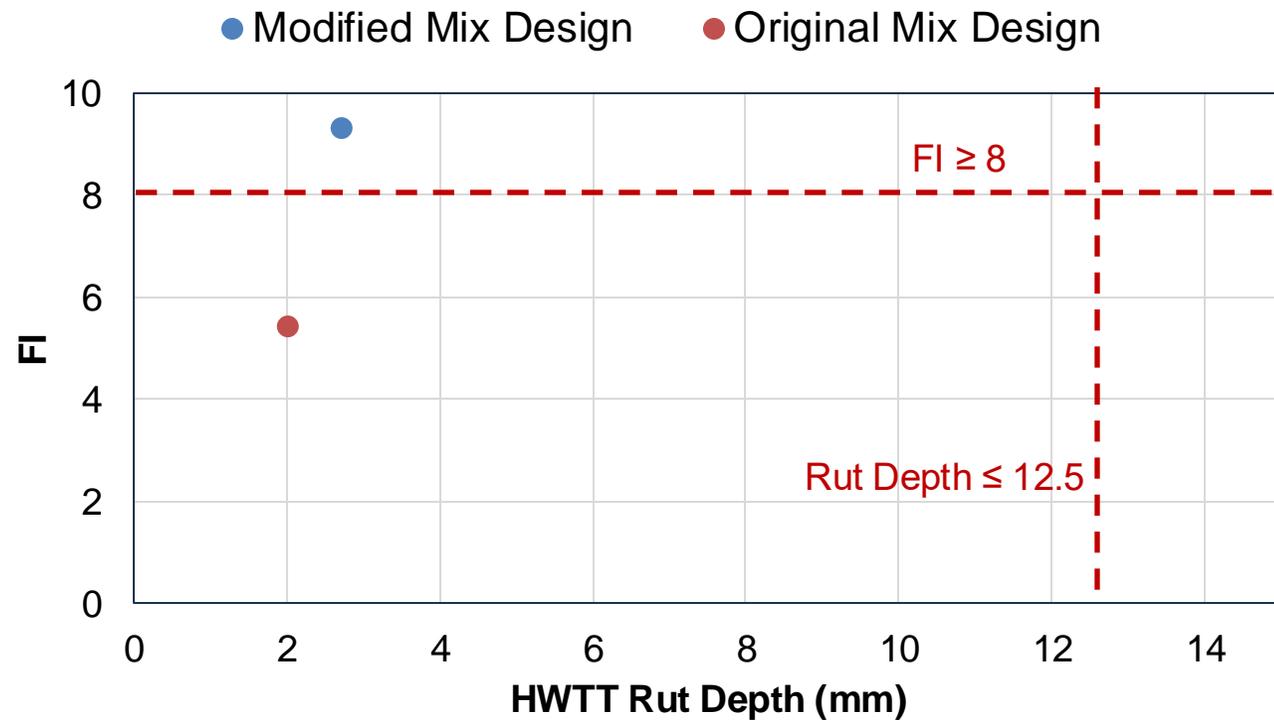
- Solution:
 - Using a softer PMA virgin binder (PG 64-34)
 - IDOT requires ***Approach-A***
 - Softer virgin binder will not affect the volumetrics
 - PG 70-28 (PMA) had minimal rutting in HWTT



Illinois

BMD Test Parameter	Average	IDOT BMD Spec. (Average)	Pass/Fail
HWTT Rut Depth at 15,000 Passes (mm)	2.7	≤ 12.5	Pass
I-FIT FI	9.3	≥ 8.0	Pass

Performance space diagram →



Wisconsin

- Problem Statement:

- VMD with inadequate rutting and moisture resistance
- 6.4% PG 52S-34 OBC (unmodified)



BMD Test Parameter	Average	WisDOT BMD Spec. (Average)	Pass/Fail
HWTT N _{12.5} (passes)	6400	≥10,000	Fail
HWTT SIP (passes)	4250	≥8,000	Fail
IDEAL-CT CT _{Index}	60.6	≥30.0	Pass

- Solution:

- Using a PMA binder PG 58H-34 and adding a liquid anti-strip additive
 - *WisDOT requires **Approach-A** and LAS additive improves HWTT results*

Volumetric Mix Design
Polymer Modified Asphalt

Wisconsin Department Of Transportation
Hamburg Wheel Tracking Test



Wisconsin

- LAS Additive

BMD Test Parameter	Average	WisDOT BMD Spec. (Average)	Pass/Fail
HWTT N _{12.5} (passes)	8100	≥10,000	Fail
HWTT SIP (passes)	5750	≥8,000	Fail

- PG 58H-34 PMA binder

BMD Test Parameter	Average	WisDOT BMD Spec. (Average)	Pass/Fail
HWTT N _{12.5} (passes)	11400	≥10,000	Pass
HWTT SIP (passes)	8100	≥8,000	Marginal Pass

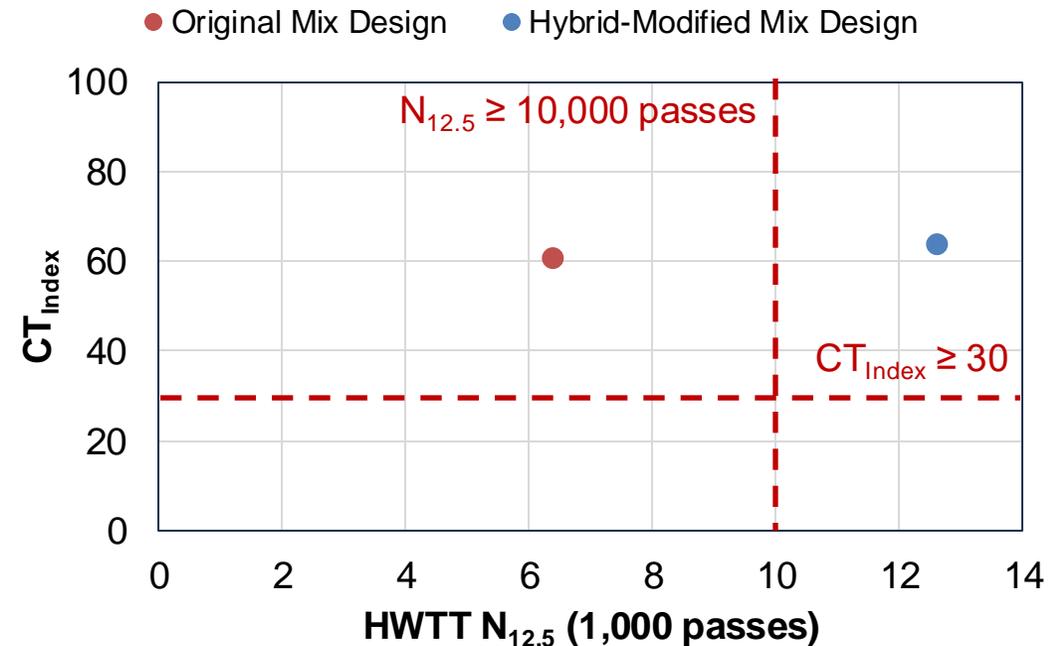
- LAS Additive + PG 58H-34 PMA binder (Hybrid)

BMD Test Parameter	Average	WisDOT BMD Spec. (Average)	Pass/Fail
HWTT N _{12.5} (passes)	12600	≥10,000	Pass
HWTT SIP (passes)	9000	≥8,000	Pass
IDEAL-CT CT _{Index}	63.6	≥30.0	Pass



Wisconsin

- Both **mix designs were identical**, except for a PMA binder and a LAS additive
- The **hybrid modification** approach significantly **improved the rutting and moisture resistance** of the original mix design while maintaining cracking resistance
- Higher material cost can be justified from a life-cycle cost perspective



Alabama

- Problem Statement:

- VMD with inadequate cracking resistance

BMD Test Parameter	# Replicate	Average	Standard Deviation	ALDOT BMD Spec. (Average)	Pass/Fail
HT-IDT Strength (psi)	3	38.7	3.8	≥20	Pass
IDEAL-CT CT _{Index}	6	27.6	4.5	≥55	Fail

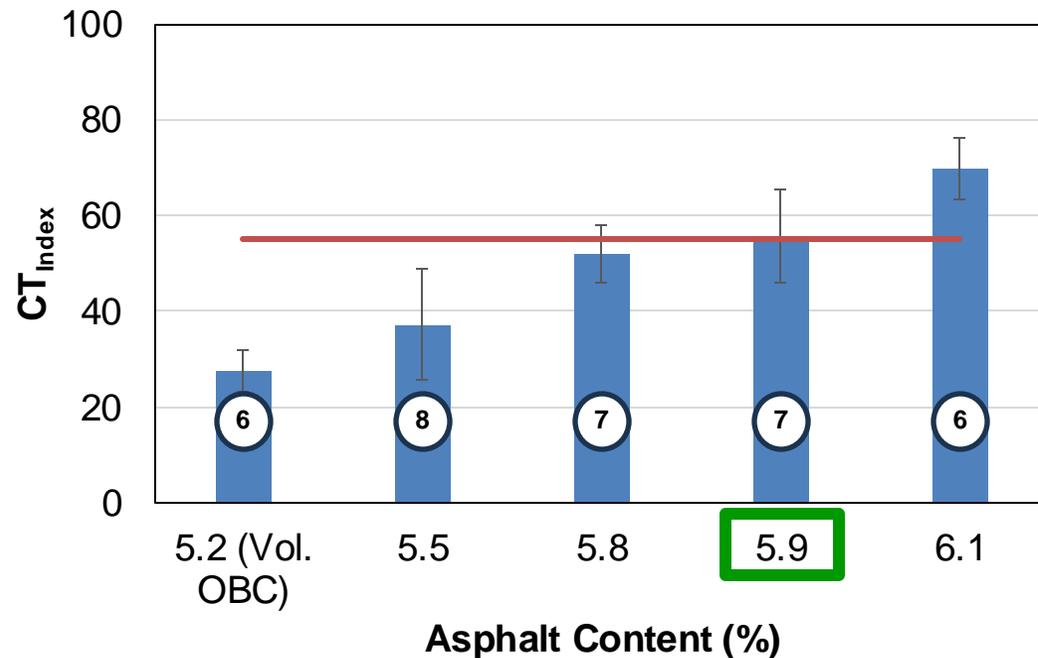


- Solution:

- Modification to meet the ALDOT BMD specs: (**Approach-C/D**)
 1. *Increasing asphalt binder content (Virgin Binder PG 67-22)*
 2. *Using WMA additives to lower mix production temperature and increasing asphalt binder content*

Alabama

1. Increasing asphalt binder content:



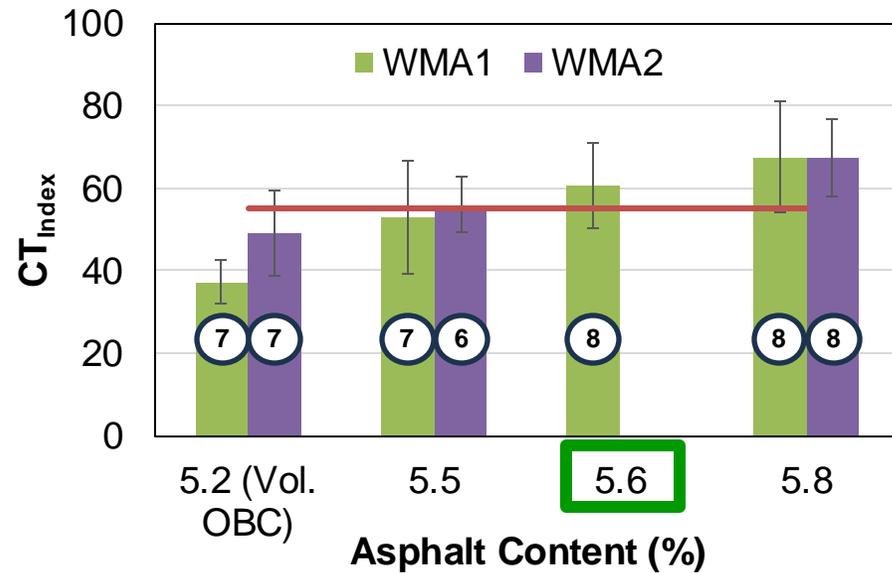
- All the other mix components and **proportions are unchanged**
- **≥6** replicates
- At 5.9% AC, **VMA = 14.5%** and **V_a = 1.8%**

BMD Test Parameter	# Replicate	Average	Standard Deviation	ALDOT BMD Spec. (Average)	Pass/Fail
HT-IDT Strength (psi)	5	27.4	2.7	≥20	Pass
IDEAL-CT CT _{Index}	7	55.7	9.9	≥55	Pass



Alabama

2. Using WMA additives and increasing binder content:



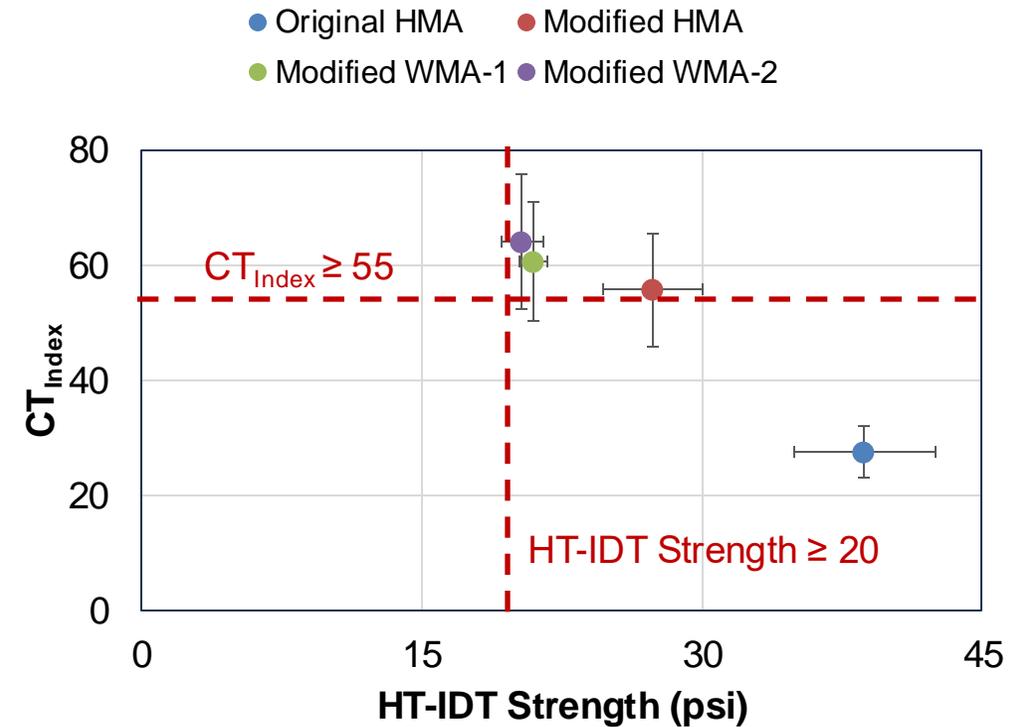
- All the other mix components and **proportions are unchanged**
- **≥6** replicates

Mix Type	BMD Test Parameter	# Replicate	Average	Standard Deviation	ALDOT BMD Spec. (Average)	Pass/Fail
WMA1	HT-IDT Strength (psi)	3	21	0.7	≥20	Pass
	IDEAL-CT CT _{Index}	8	60.6	10.4	≥55	Pass
WMA2	HT-IDT Strength (psi)	3	20.4	1.1	≥20	Pass
	IDEAL-CT CT _{Index}	8	64	11.8	≥55	Pass



Alabama

Mix Property	Original HMA	Modified HMA	Modified WMA-1	Modified WMA-2
Total Binder Content (%)	5.2	5.9	5.6	5.6
RAP Content (%)	20	20	20	20
Additive	-	-	WMA-1	WMA-2
RAP Binder Replacement (%)	21%	19%	20%	20%
Virgin Binder Content (%)	4.1	4.8	4.5	4.5
Virgin Binder Grade	PG 67-22	PG 67-22	PG 67-22	PG 67-22
Air Voids (%)	3.9	1.8	2.7	2.7
VMA (%)	14.7	14.5	14.5	14.5
VFA (%)	74	87	81	81

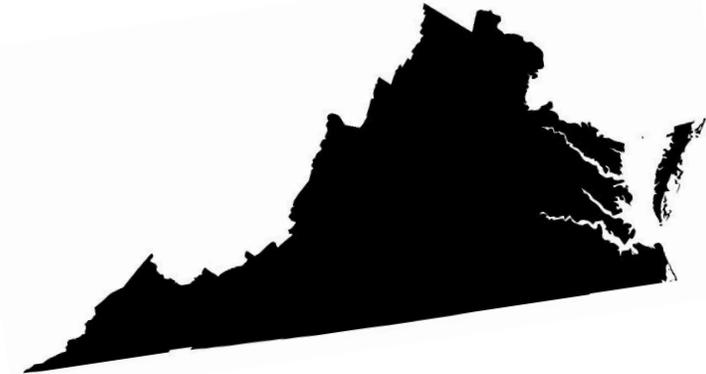


Virginia

- Problem Statement:

- VMD with inadequate cracking resistance

BMD Test Parameter	Average	VDOT BMD Spec. (Average)	Pass/Fail
APA Rut Depth (mm)	2.7	≤8.0	Pass
IDEAL-CT CT _{Index}	45	≥70	Fail
Cantabro Mass Loss (%)	5.2	≤7.5	Pass



- Solution:

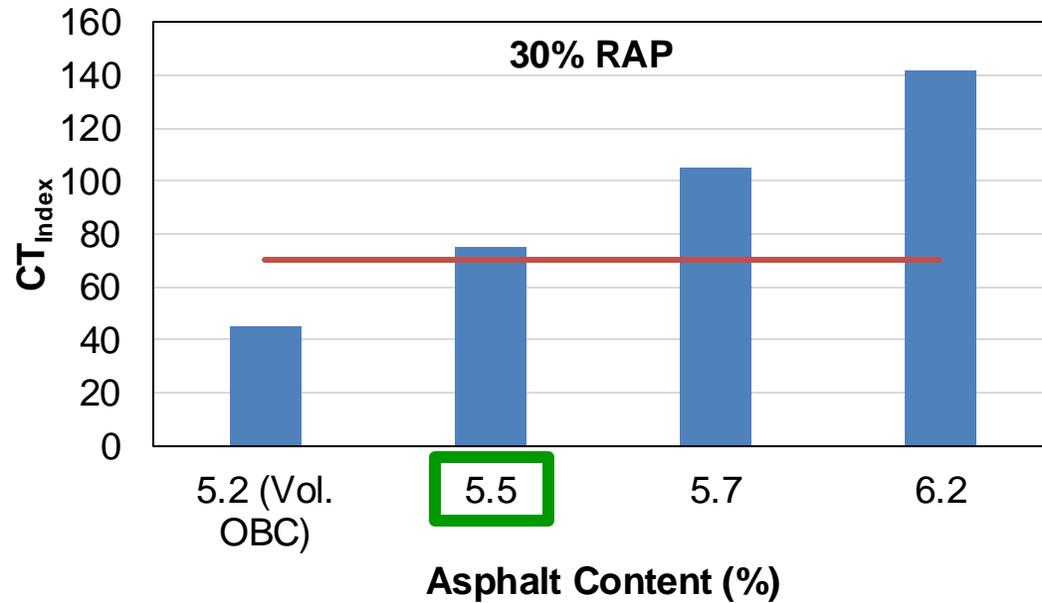
- Modification to meet the VDOT BMD specs: (**Approach-C**)

1. *Increasing asphalt binder content (Virgin Binder PG 64-22)*
2. *Increasing RAP content, adding a recycling agent, and increasing asphalt binder content*



Virginia

1. Increasing asphalt binder content:



- 9.5 mm NMAS surface mix
- All the other mix components and **proportions are unchanged**
- At 5.5% AC, **VMA = 16.2%** and **V_a = 2.9%** instead of 16.3% and 4% respectively at 5.2% AC originally

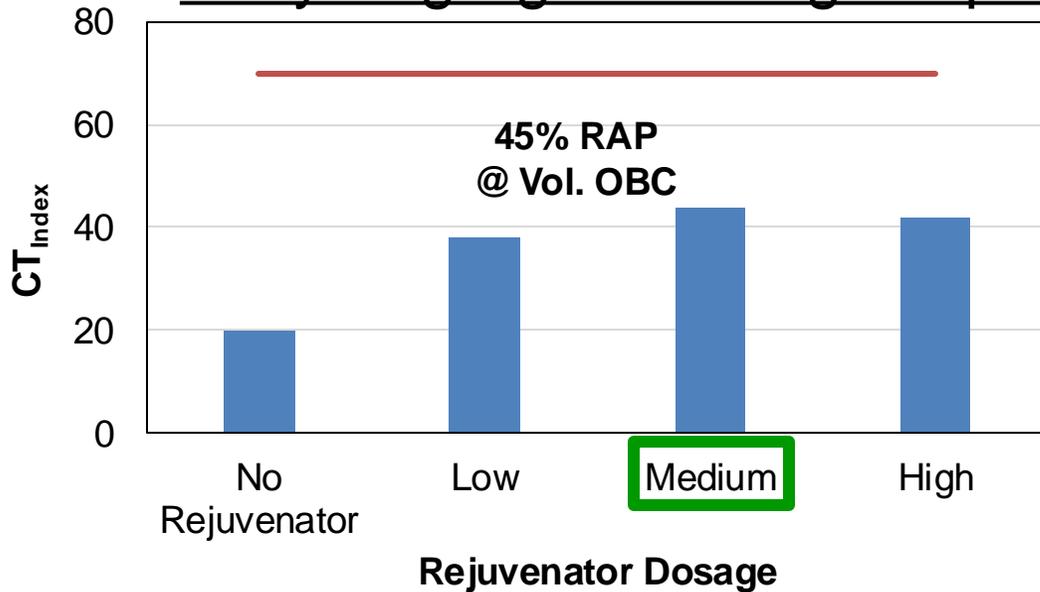
BMD Test Parameter	Average	VDOT BMD Spec. (Average)	Pass/Fail
APA Rut Depth (mm)	3.3	≤8.0	Pass
IDEAL-CT CT _{Index}	75	≥70	Pass
Cantabro Mass Loss (%)	4.7	≤7.5	Pass



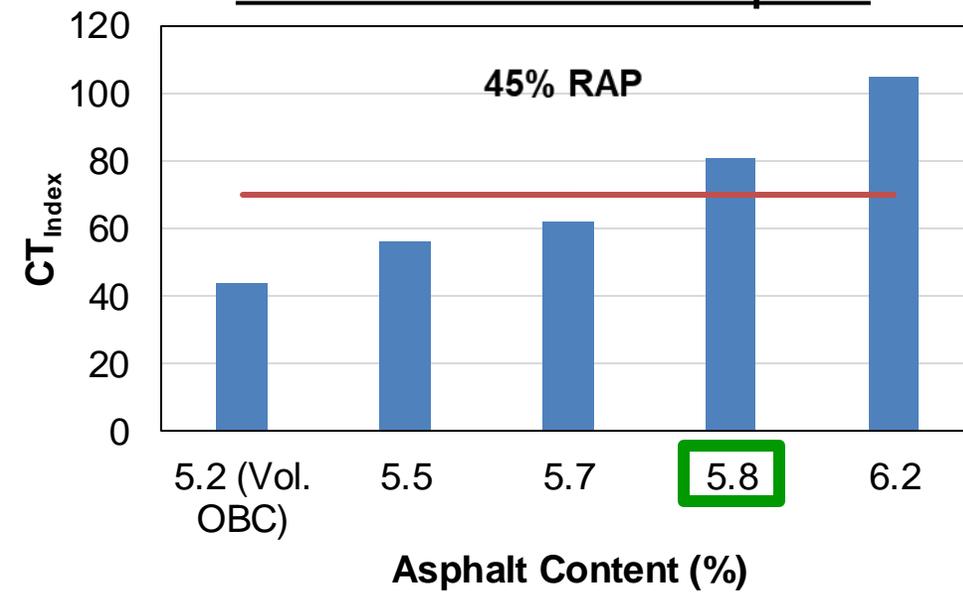
Virginia

2. Increasing RAP and binder content + adding recycling agent

Recycling Agent Dosage Impact



Binder Content Impact

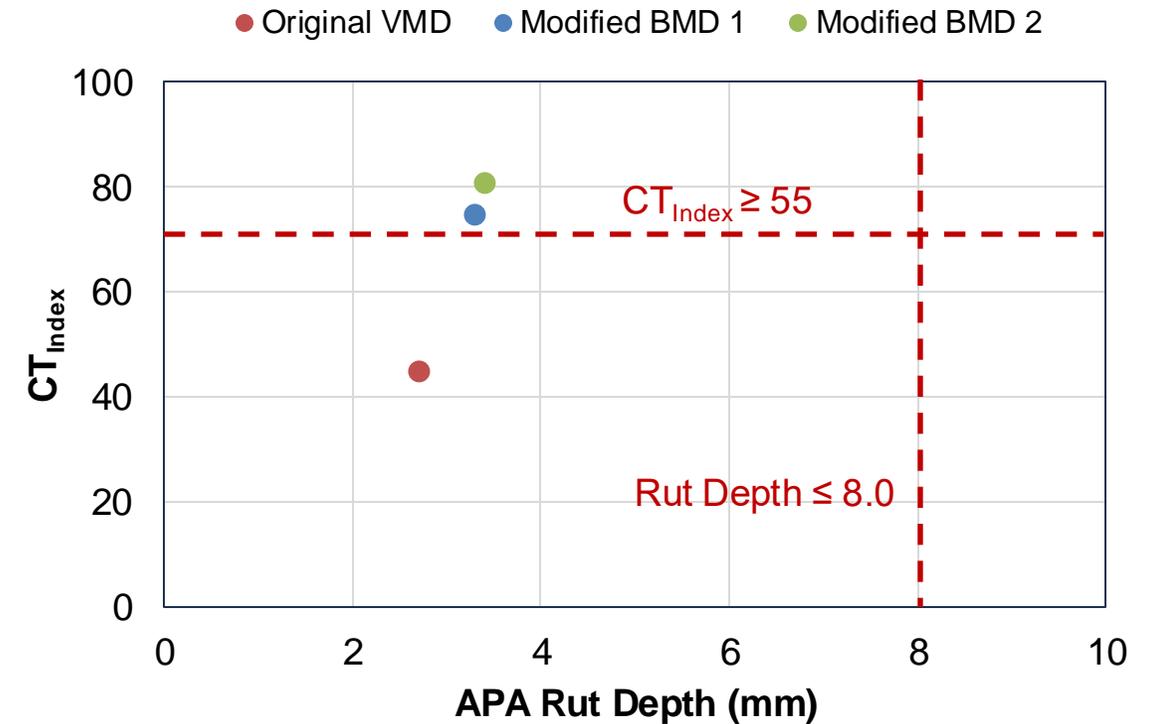


BMD Test Parameter	Average	VDOT BMD Spec. (Average)	Pass/Fail
APA Rut Depth (mm)	3.3	≤8.0	Pass
IDEAL-CT CT _{Index}	75	≥70	Pass
Cantabro Mass Loss (%)	4.7	≤7.5	Pass



Virginia

Mix Property	Original VMD	Modified BMD 1	Modified BMD 2
Total Binder Content (%)	5.2	5.5	5.8
RAP Content (%)	30	30	45
Additive	-	-	Recycling Agent
RAP Binder Replacement (%)	24	24	38
Virgin Binder Content (%)	3.9	4.2	3.6
Virgin Binder Grade	PG 64-22	PG 64-22	PG 64-22
Air Voids (%)	4	2.9	2.3
VMA (%)	16.3	16.2	16.5
VFA (%)	75.5	82.1	86.1



Critical Considerations and Research Efforts on BMD



Critical Considerations to BMD



Material Screening and Selection

- Materials (RAP, RAs, and binders) should be carefully screened and selected to ensure their **compatibility** and **pavement longevity**



Test Simplification and Efficiency

- Test methods should be **flexible, simple, and accommodating to location** and production variables
- Should be validated: Specimen and pavement scale



QA Process

- **Quick access to performance properties** is crucial for allowing contractors to adjust production variables without delays

Examples of On-going Research Efforts to Support BMD Implementation

- BMD development for airfield asphalt mixtures
- Practicality improvements for Illinois Flexibility Index (I-FIT) test
- Development of Simplified Wedge Splitting Test (SWST)

BMD Application for Airfield Asphalt Pavement

Objectives:

- Introduce cracking test(s) that measure fundamental properties
 - Rapid, low-cost, minimal training, and easy to incorporate in QC/QA
 - Consider crack initiation and propagation
- Correlate the recommended test(s) to pavement performance at:
 - National Airport Pavement Test Facility (NAPTF)
 - National Airport Pavement and Materials Research Center (NAPMRC)

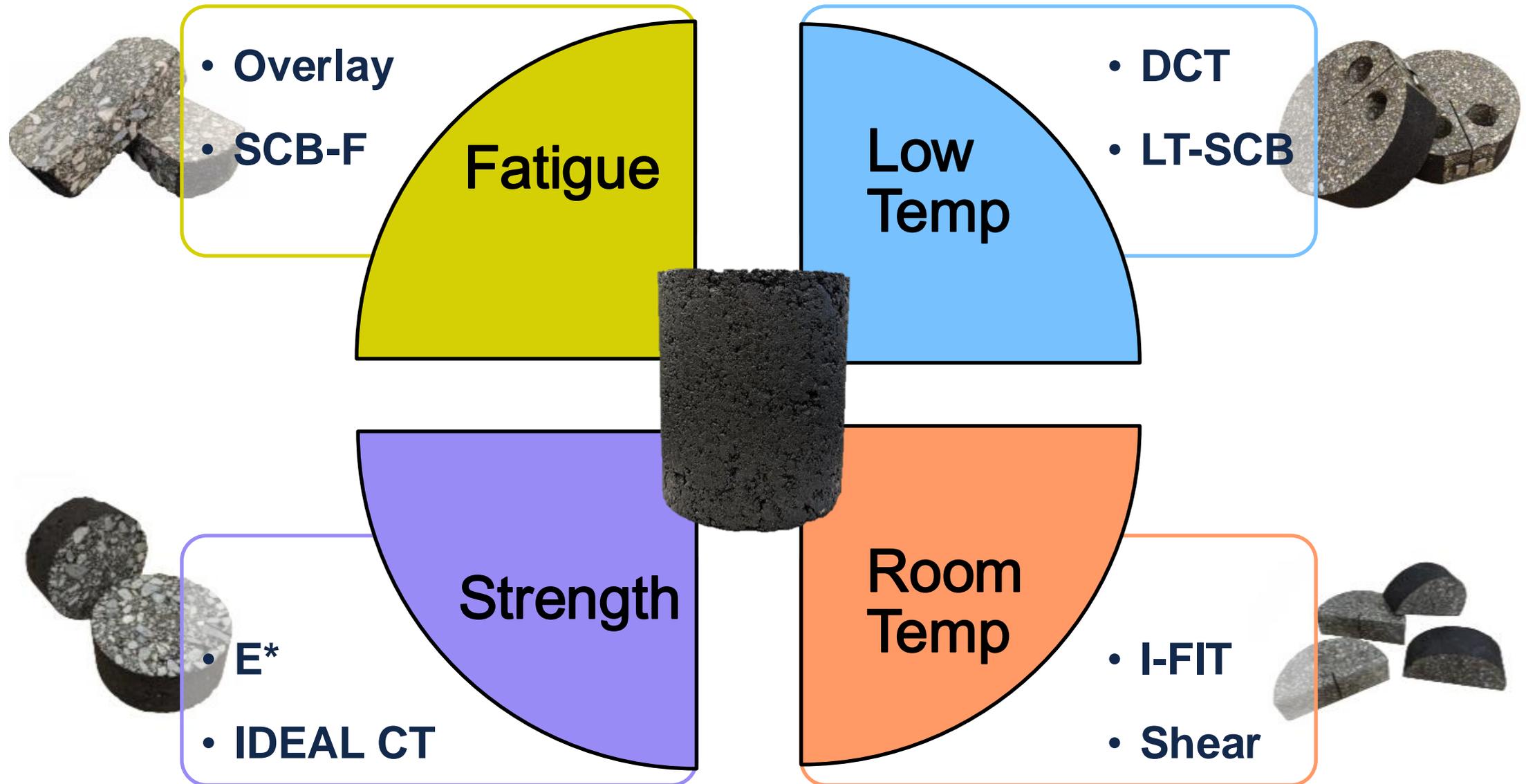


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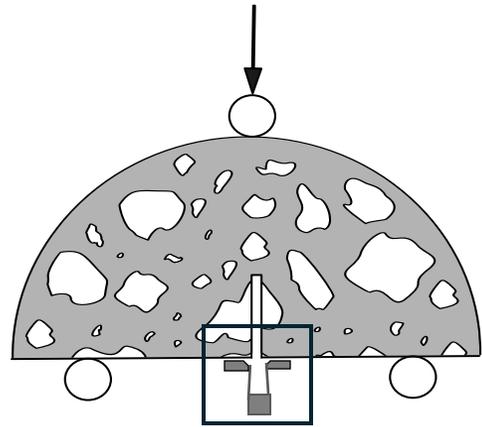
AIRPORT ASPHALT PAVEMENT TECHNOLOGY PROGRAM (AAPTP)



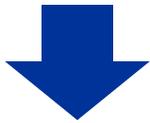
Cracking Test Evaluation



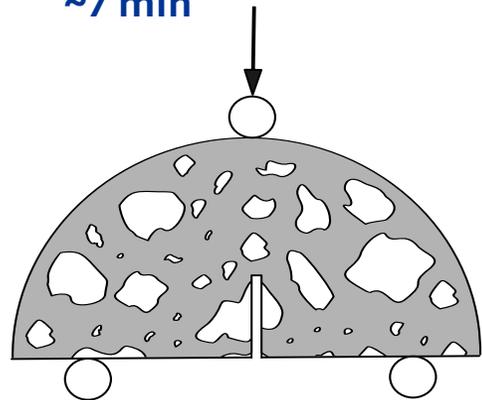
Low-Temperature SCB Test Simplification (ongoing)



~1 hr

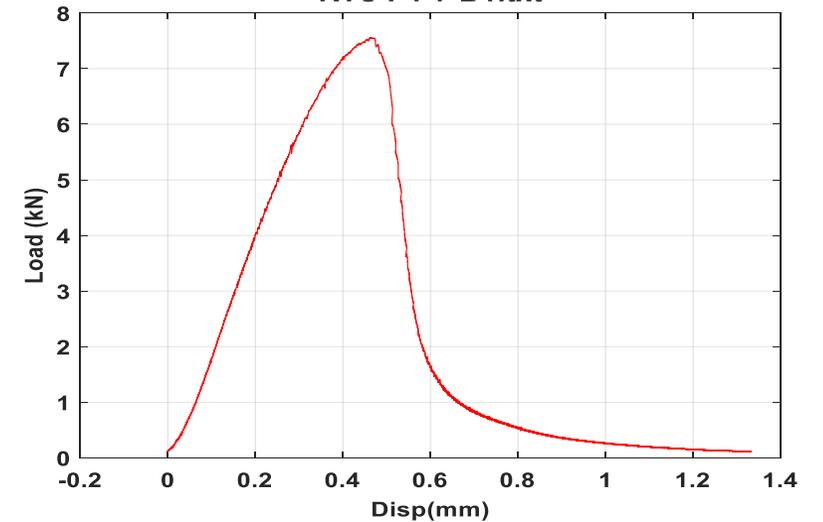


~7 min



■ Protocols

- Specimen = ~~24.7 mm~~ 50 mm
- Test Temp = ~~PGL~~ True PGL(E) + 10° C
- Control = ~~CMOD~~ LLD
- Rate = ~~0.03~~ 0.15 mm/min

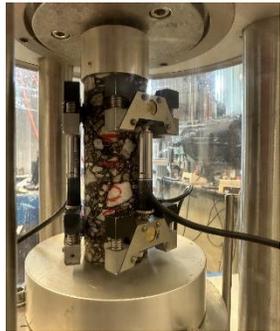


Examples of On-going Research Efforts to Support BMD Implementation

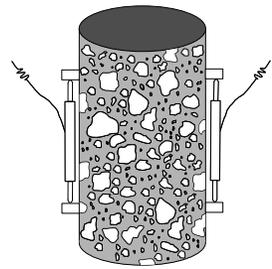
- BMD development for airfield asphalt mixtures
- **Practicality improvements for Illinois Flexibility Index test (I-FIT)**
- Development of Simplified Wedge Splitting Test (SWST)

Test Temperature and Test Simplification for Illinois Flexibility Index Test (I-FIT) (AASHTO T-393)

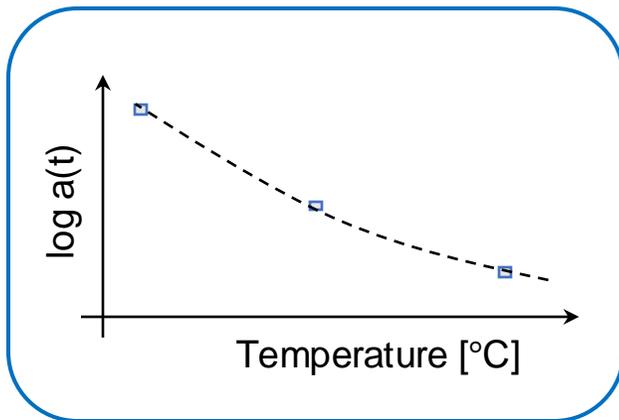
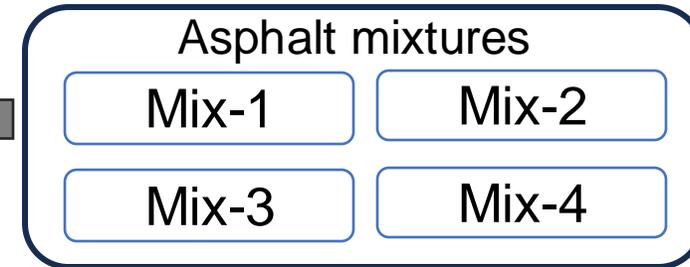
- AASHTO T-393 uses single test temperature for all mixtures (25 °C)
- It needs ability to carefully control specimen temperature



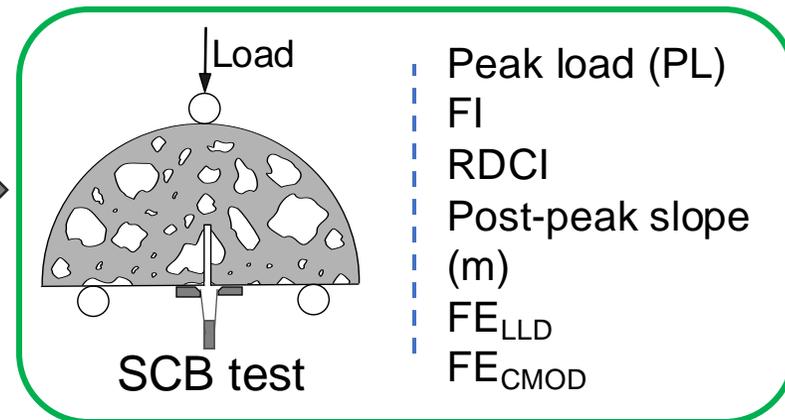
AASHTO T 342



Complex modulus
(small size samples)
- 2.9, 18 and 30°C
- 0.1, 0.5, 1, 5, 10
and 25 Hz



TTSP
Five loading rate
and temperature
combinations



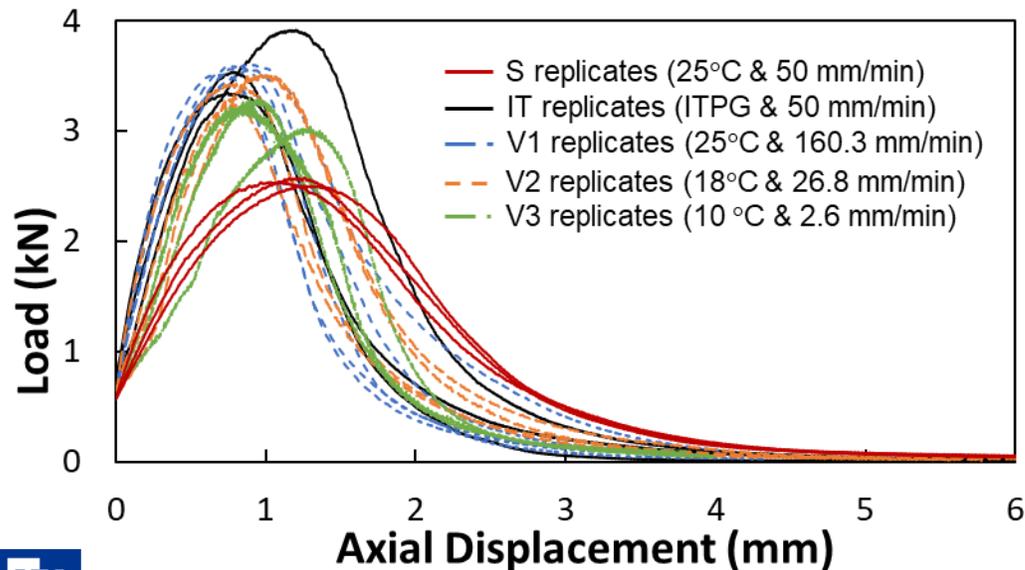
Peak load (PL)
FI
RDCI
Post-peak slope
(m)
 FE_{LLD}
 FE_{CMOD}



AASHTO T 393

I-FIT Test Conditions: Summary and Results

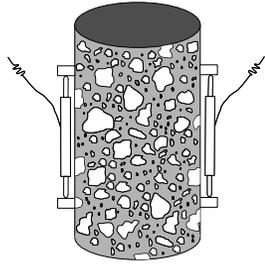
Variation	Mix-1		Mix-2		Mix-3		Mix-4	
	T (°C)	LLD (mm/min)	T (°C)	LLD (mm/min)	T (°C)	LLD (mm/min)	T (°C)	LLD (mm/min)
Standard (S)	25.0	50.0	25.0	50.0	25.0	50.0	25.0	50.0
ITPG scenario (IT)	20.4	50.0	30.0	50.0	29.8	50.0	28.1	50.0
Variation-1 (V1)	25.0	160.3	25.0	14.5	25.0	14.9	25.0	22.5
Variation-2 (V2)	18.0	26.8	18.0	2.1	18.0	2.0	18.0	3.0
Variation-3 (V3)	10.0	2.6	34.0	127.3	34.0	127.0	34.0	213.2



Study Outcomes:

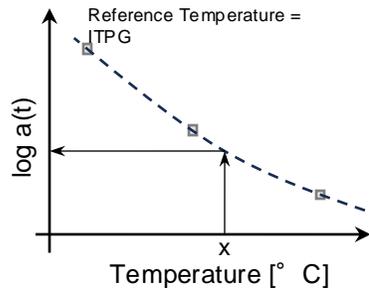
- Test temperature should be related to required PG grade for the pavement (based on pavement location and depth within pavement)
- Test can be run at room temperatures without temperature conditioning and loading rate is adjusted based on temperature

Implementation Procedures for Agencies



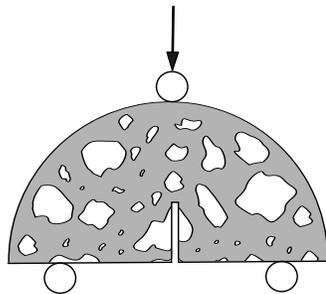
Step-1 (Complex Modulus Test)

Perform the complex modulus test with test range covering the ITPG of the binder.



Step-2 (Shift Factor Calculation)

Determine the shift factor [a(t)] for the target SCB test temperature [x] from the master curve at ITPG as a reference temperature (based on complex modulus test results).



Step-3 (SCB Test)

Determine the adjusted loading rate required for target SCB test temperature [x]

$$\text{[i.e. } \frac{\text{Standard loading rate}}{\text{Shift factor}} = \frac{50}{[a(t)]} \text{ mm/min].}$$

Region specific tables can be prepared having temperature and loading rate



Application of time-temperature superposition principle for cracking characterization of asphalt mixtures using semi-circular bend test

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ARTICLE INFO

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Reclaimed asphalt pavement
Cracking characterization
Flexibility index
Balanced mix design

ABSTRACT

In recent decades, numerous laboratory tests have been developed to characterize the cracking performance of asphalt mixtures. Among these, the semi-circular bend (SCB)-based Illinois flexibility index test (I-FIT) (AASHTO T 393) has emerged as a favored method due to its straightforward specimen preparation and better sensitivity to mixture variables. However, the cracking susceptibility of asphalt mixtures is significantly influenced by temperature and loading rate, leading to a discrepancy in performance between standard test conditions and real-world scenarios. To address this challenge, this study explores the applicability of the time-temperature superposition principle (TTSP) for the SCB test. The study evaluates four mixtures incorporating reclaimed asphalt pavement (RAP) and/or reclaimed asphalt shingles (RAS) across various loading rates and temperature combinations. Findings suggest that TTSP can be applied to the SCB test without significantly impacting cracking performance indices such as flexibility index and post-peak slope. Furthermore, the study also demonstrates that TTSP can be applied in the crack propagation phase during the post-peak domain of the mixtures. The findings of the study will allow laboratories to develop more flexible testing plans without compromising the accuracy and reliability of performance parameters by adjusting the loading rate corresponding to the test temperature and vice versa. Consequently, this approach can potentially make a balanced mix design (BMD) practice more implementable.

1. Introduction

The semi-circular bend (SCB) test has been developed to measure the fracture properties of asphalt mixtures, particularly in the domain of low and intermediate temperatures as illustrated in Fig. 1a. The SCB test is widely used for both asphalt mixture design and quality control due to its straightforward specimen preparation procedure and equipment availability in laboratories. Its practicality and relatively quick testing time are a few of the reasons why it is widely used by agencies and contractors. It is extensively employed to evaluate the effects of fibers [1] and recycled materials, such as reclaimed asphalt pavement (RAP) [2,3], reclaimed asphalt shingles (RAS) [4,5], and crumb rubber [6,7]

on mixture performance. Additionally, the notched SCB specimens can potentially be used to predict indirect tensile strength in the low-temperature domain [8,9].

The test results, i.e., the load vs. load-line displacement (LLD) curve (Fig. 1b), can be used to evaluate several parameters, including peak load or strength, flexibility index (FI), pre-peak fracture energy, post-peak slope, fracture energy (FE), and rate-dependent cracking index (RDCI). These parameters are crucial for assessing the cracking resistance of asphalt materials under standard testing conditions (displacement rate and test temperature) [10]. Additionally, the cracking performance of the mixtures can be evaluated using the load vs. crack mouth opening displacement (CMOD) curve if CMOD is measured

Abbreviations: BMD, balanced mix design; CMOD, crack mouth opening displacement; COV, coefficient of variation; FE, fracture energy; FI, flexibility index; HMA, hot mix asphalt; HTPG, high-temperature performance grade; IDBAL-CT, indirect tensile asphalt cracking test; I-FIT, Illinois flexibility index test; ITPO, intermediate temperature performance grade; LLD, load-line displacement; LTPO, low-temperature performance grade; LVE, linear viscoelastic; MEPDG, mechanistic-empirical pavement design guide; NMAAS, nominal maximum aggregate size; PG, performance grade; PRI, performance ranking index; RAP, reclaimed asphalt pavement; RAS, reclaimed asphalt shingles; RDCI, rate-dependent cracking index; RLDP, repeated load permanent deformation; S, standard scenario; SCB, semi-circular bend; TTSP, time-temperature superposition principle; V1, variation-1; V2, variation-2; V3, variation-3; WLF, Williams-Landel-Ferry.

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More Details

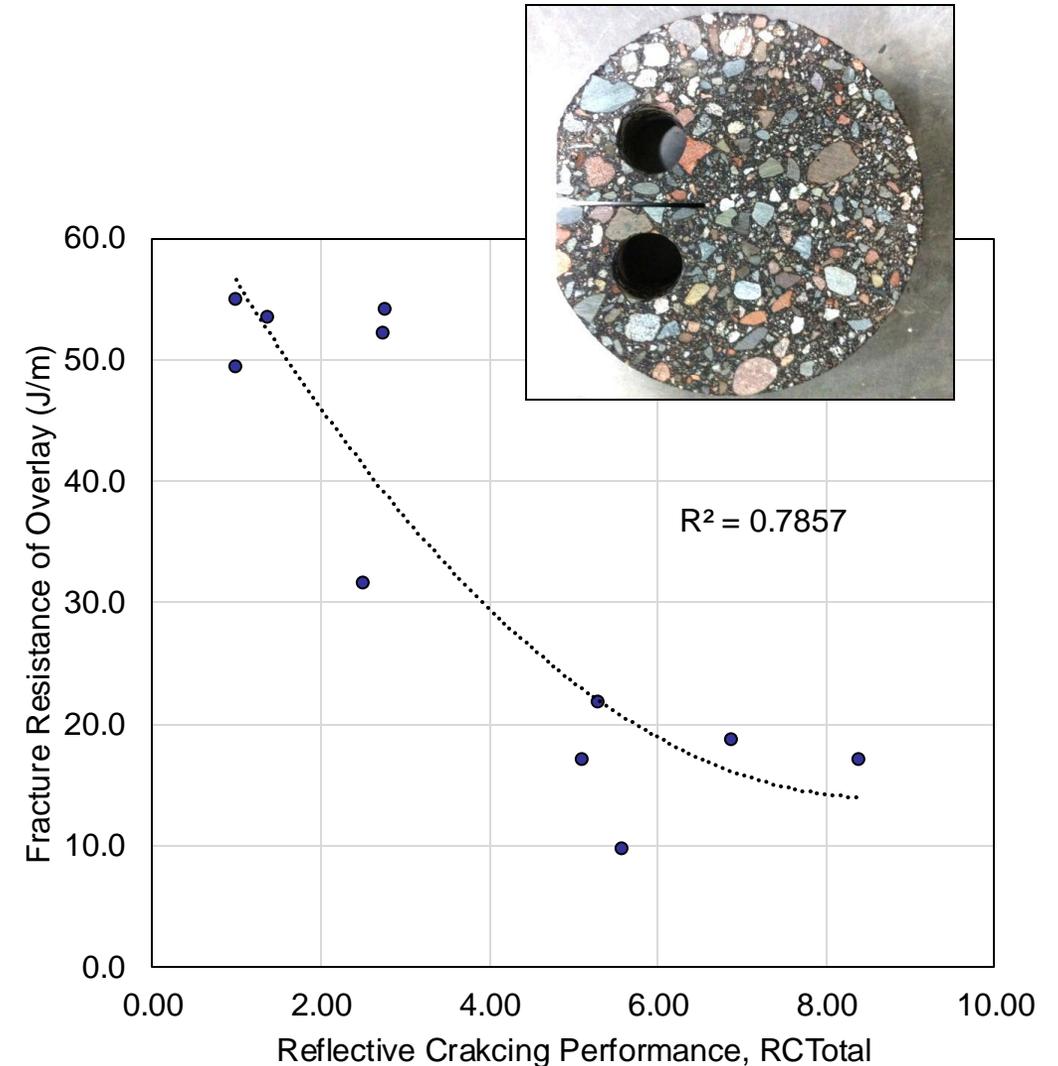


Examples of On-going Research Efforts to Support BMD Implementation

- BMD development for airfield asphalt mixtures
- Practicality improvements for Illinois Flexibility Index (I-FIT) test
- **Development of Simplified Wedge Splitting Test (SWST)**
 - **Acknowledgements: Shongtao Dai and Joseph Voels (MnDOT)**

Simplified Wedge Splitting Test (SWST): Motivation

- DCT has shown to correlate very well with transverse cracking performance of asphalt mixtures in Minnesota
 - Fracture energy (G_f) as well as post-peak indices have been assessed in past and will get more comprehensive assessment under a new MnDOT/LRRB effort.
 - A massive database of results exist for MN mixes tested using DCT
- DCT is a complex test: specimen fabrication, specimen conditioning, testing
- DCT cannot be used as a routine test within a QA process for asphalt mixtures



How to combine simplicity and existing knowledge?

- DCT:
 - Complex specimen geometry → Needs significant operator training
 - Takes too long to get results → Not good for process control
- DCT + CT-Index → Simplified Wedge Splitting Test (SWST)
- SWST is not an entirely new concept



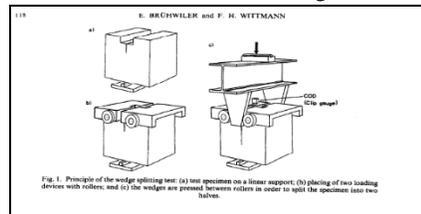
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THE WEDGE SPLITTING TEST, A NEW METHOD OF PERFORMING STABLE FRACTURE MECHANICS TESTS

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Abstract—The wedge splitting test is a new test method to perform stable fracture mechanics tests on concrete and concrete-like materials. Specific fracture energy G_f , as well as fracture toughness K_{Ic} , are determined using simple specimens like cubes or cylinders. The main features of the wedge splitting test are described and compared to other test methods. Identical G_f -values are found irrespective of the test method and the specimen shape. The significance of the interaction between the testing standard, the test controller, the test method and the material properties for the performance and interpretation of stable fracture tests is outlined.



THE WEDGE SPLITTING TEST, A NEW METHOD OF PERFORMING STABLE FRACTURE MECHANICS TESTS

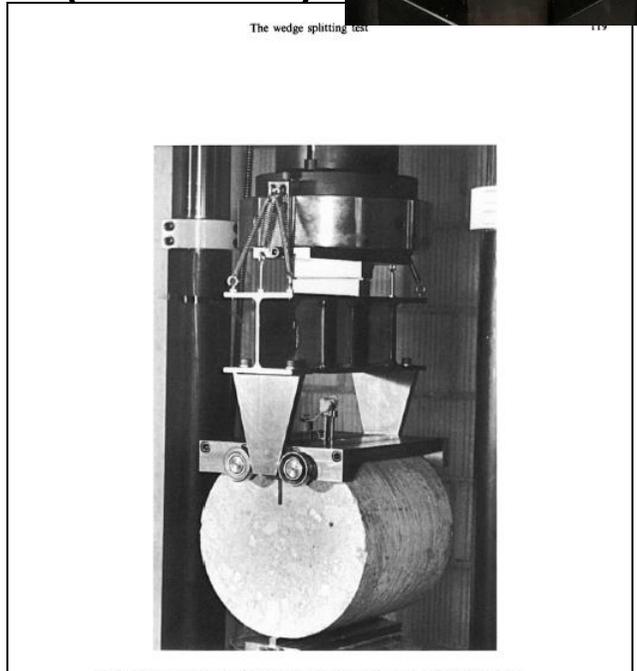
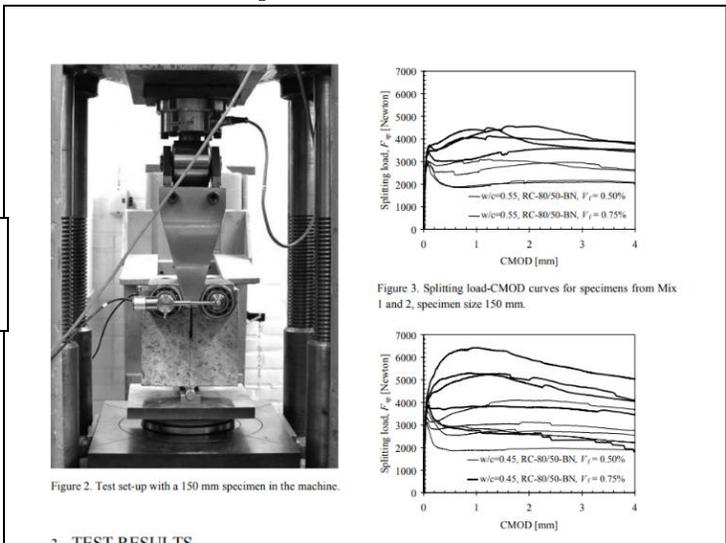
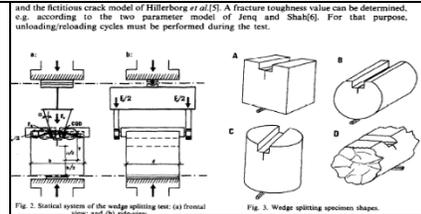
specific fracture energy G_f [1]. This recommendation specifies a testing method consisting of stable three point bending tests (TPBT) on notched beams. Recently, the compact tension test (CT-test) was used for the determination of the specific fracture energy G_f of concrete[2]. Both test methods however have drawbacks with regard to future use as standard tests.

The aim of this article is to present the wedge splitting test, a new method of performing stable fracture mechanics tests on concrete and concrete-like materials[3]. First, the proposed test method is described and special features are outlined. Then, G_f -values as determined by means of different test methods are reported, and finally, the stability of fracture tests is discussed.

2. THE WEDGE SPLITTING TEST

2.1. Description of the test method

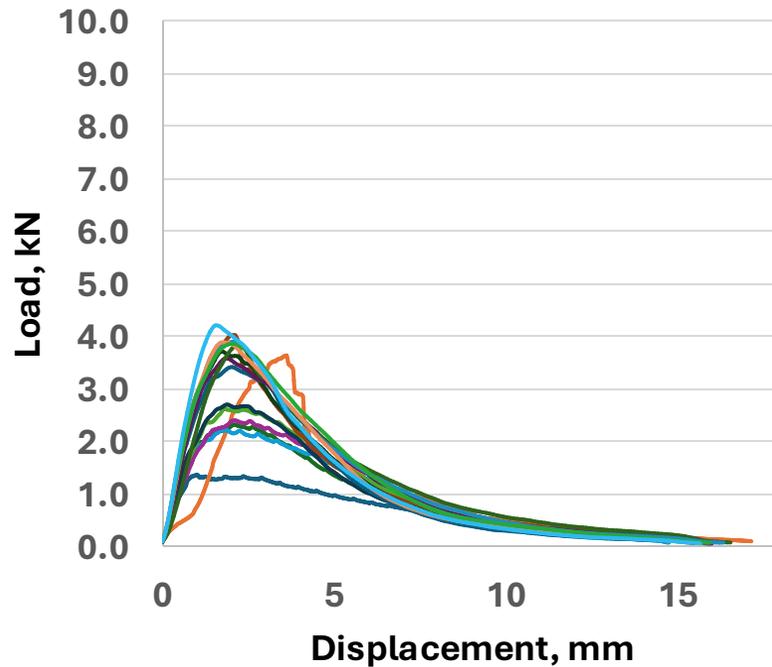
The set-up of the wedge splitting test is presented schematically in Fig. 1. First, a specimen is prepared by sawing or casing a groove and a notch. This specimen is placed on a linear support, which is fixed on the lower plate of the testing machine (Fig. 1a). Two massive loading devices both equipped with rollers, are placed on the top of the specimen (as shown in Fig. 1b). A stiff steel profile with two identical wedges is fixed at the upper plate of the testing machine,



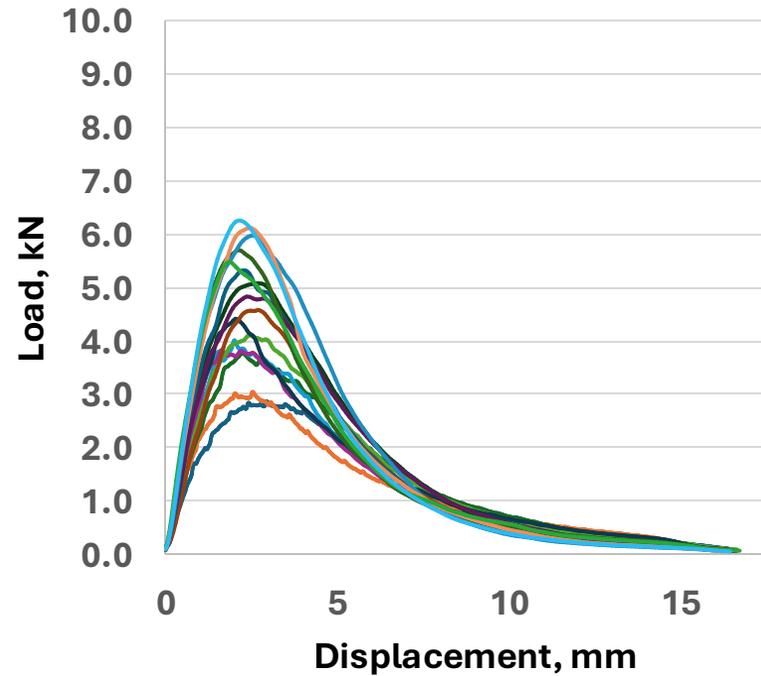
MnDOT Testing using SWST

- Different notch depths and loading rates (to compensate for test temperatures)

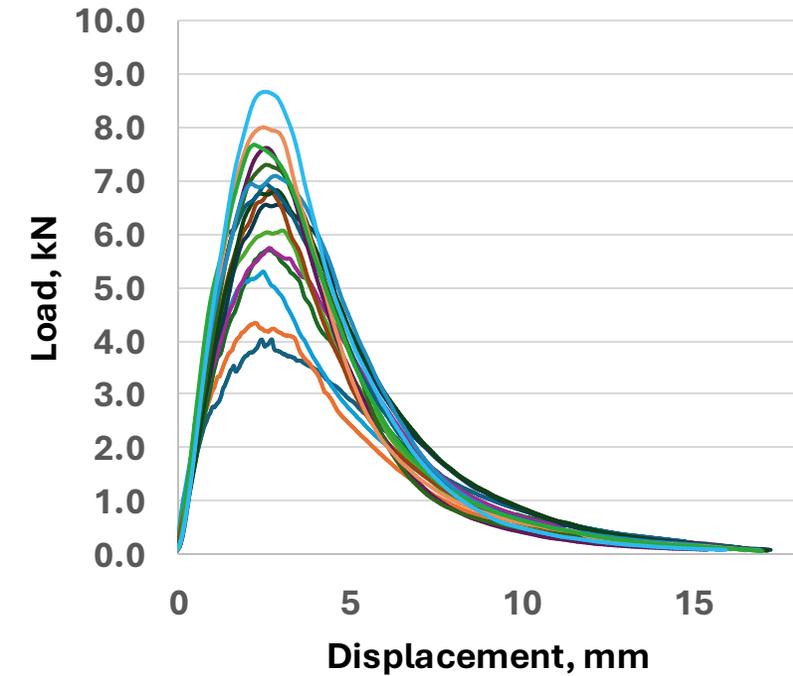
Ligament length = 65 mm



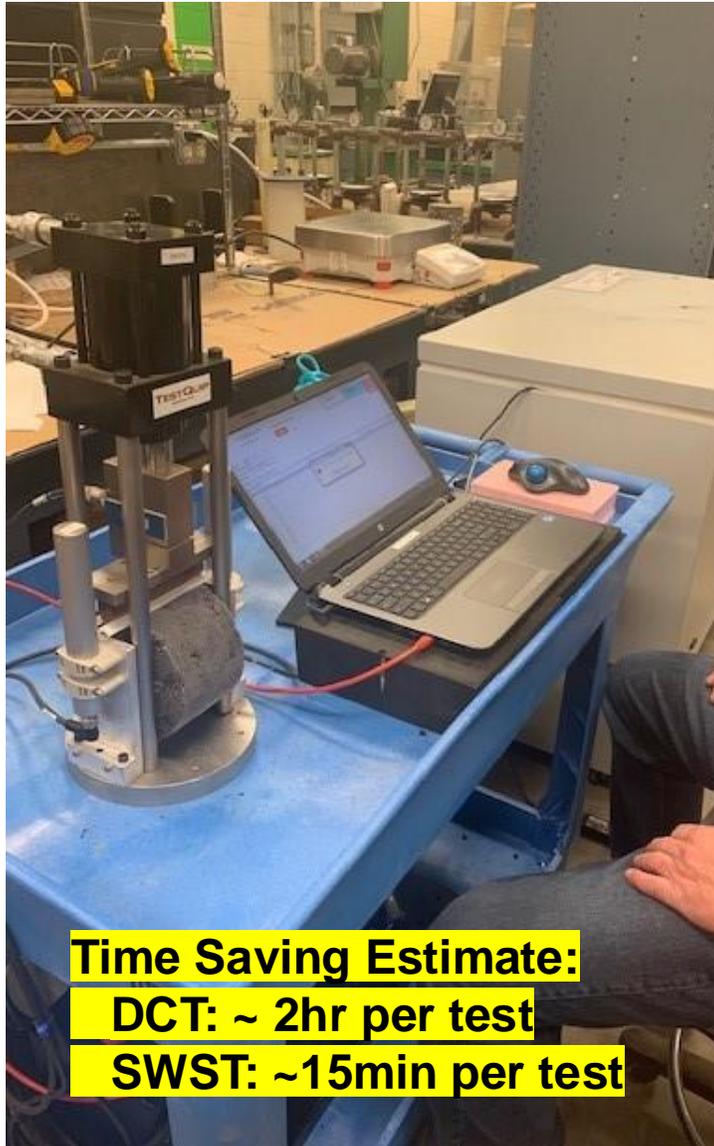
Ligament length = 75 mm



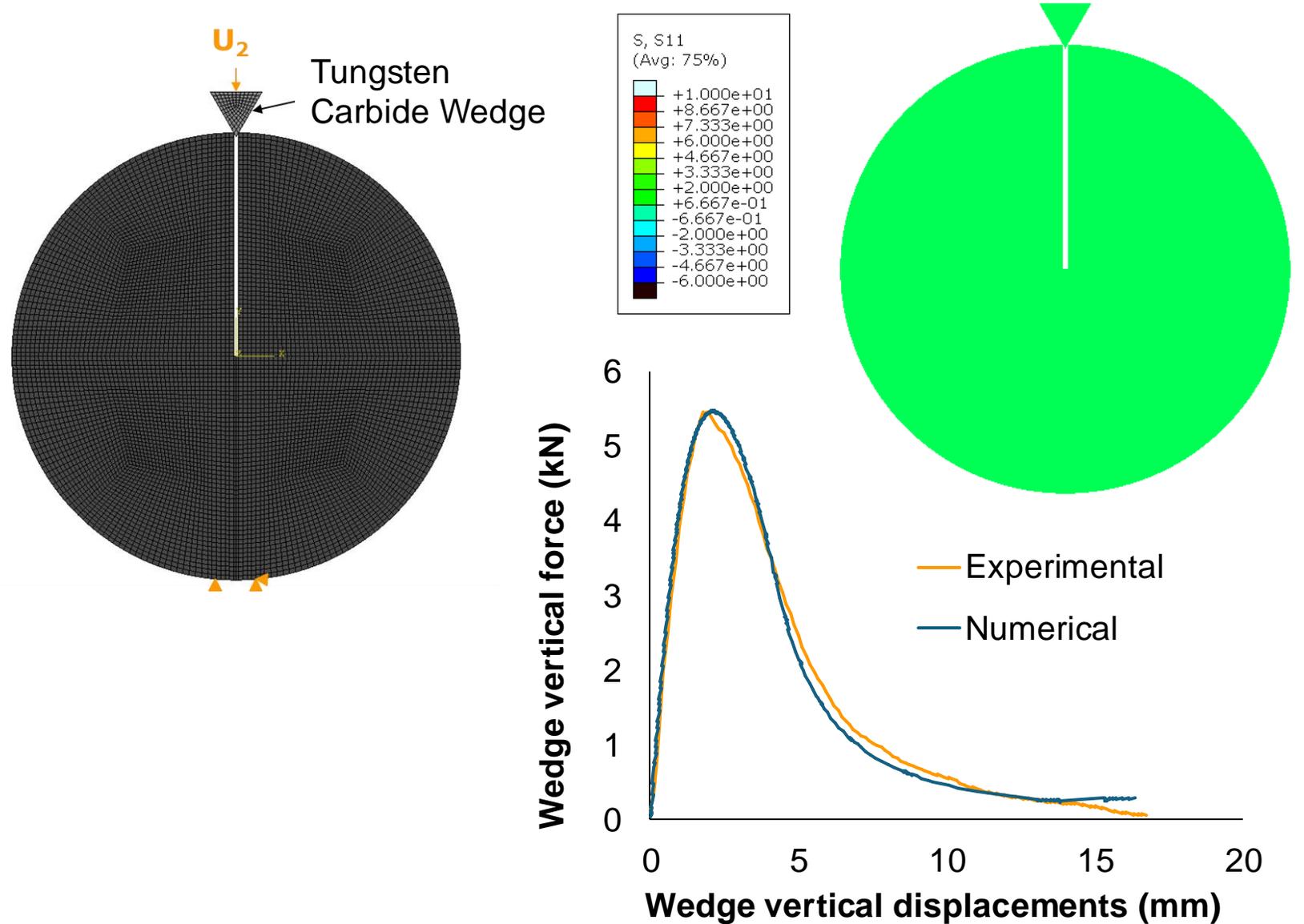
Ligament length = 85 mm



SWST Trials and Modelling



Time Saving Estimate:
DCT: ~ 2hr per test
SWST: ~15min per test



BALANCED MIX DESIGN RESOURCE GUIDE

- APPROACHES
- TESTS
- IMPLEMENTATION
- RESOURCES
- TOOLS
- WORKING GROUP

What is Balanced Mix Design?

Balanced Mix Design (BMD) is defined as "asphalt mix design using performance tests on appropriately conditioned specimens that address multiple modes of distress taking into consideration mix aging, traffic, climate and location within the pavement structure" per AASHTO PP 105 20. This definition was initially established by the former Federal Highway Administration (FHWA) Expert Task Group (ETG) Balanced Mix Design Task Force in 2015.



Why is Balanced Mix Design Needed?

ENSURE PERFORMANCE

Concerns with durability and cracking issues of asphalt pavements along with the growing awareness of the shortcomings of volumetric mix design systems have driven many SHAs and the asphalt pavement industry to explore the use of BMD as a new approach to asphalt mix design and production acceptance.

ENABLE INNOVATION

Establishing the state of performance of commonly used mixes (i.e., cataloging mixes) and optimizing those mixes to achieve performance will allow companies to help move asphalt related specifications forward to better ensure the needed field performance can be obtained.

ECONOMIC OPTIMIZATION

Balanced Mix Design allows for the optimization of mixes in terms of cost-effective material use (e.g., asphalt binder, aggregate, recycled material, additive, etc.) and performance. Without knowing the true performance of mixes, decisions on material use will likely be made based on assumptions, past experience (which may not hold true currently), raw cost alone or current specification limits and constraints. A total picture concept of materials plus performance provides the most benefit in terms of managing risk and increasing margin opportunity.



The National Asphalt Pavement Association would like to acknowledge and thank the National Center for Asphalt Technology for their contributions in compiling this information and their continued work to keep this information up-to-date.



BMD Resources

- [NAPA-IS 143 BMD Resource Guide 2021](#)
- [NCHRP 20-07 Task 406 BMD Framework Final Report](#)
- [NCHRP Synthesis 552 Practices for Fabricating Asphalt Specimens for Performance Testing in Laboratories](#)
- Technical Report: Positive Practices, Lessons Learned, and Challenges When Implementing Balanced Design of Asphalt Mixtures: Site Visits ([WRSC-TR-22-11](#))
- Technical Report: Index-based tests for performance-engineered mixture designs for asphalt pavements ([FHWA-HIF-19-103](#))
- BMD Case Studies



Thank you for your attention!

Questions and Comments?

